

# Performance based concrete design: Actual and future situation in Switzerland with a focus on carbonation

**fib** Hungarian Group, BME, Budapest  
Thursday, 12<sup>th</sup> October 2023

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<https://ingenieurberatung.ch/>

# Content

1. Durability: Actual Swiss situation
2. New Annex ND to SN EN 206
3. Comments to some Swiss durability tests
4. Carbonation and corrosion rate (XC3 and XC4)
5. Eurocode EC2 (2nd Generation): carbonation / concrete cover
6. Leaching as a potential risk in the future
7. Summary

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# Concrete types (buildings and civil engineering)

## Actual Swiss requirements

LV: Limiting Values; m: medium; h: high

Designation	0 (Null)	A	B	C	D	E	F	G
Exposure classes (CH)	X0	XC2	XC3	XC4, XF1	XC4, XD1, XF2	XC4, XD1, XF4	XC4, XD3, XF2	XC4, XD3, XF4
Max. w/z- / w/z <sub>eq</sub> -Wert	--	0,65	0,60	0,50	0,50	0,50	0,45	0,45
Minimum cement cont. z <sub>min</sub> , [kg/m <sup>3</sup> ]	--	280	280	300	300	300	320	320
Strength class	C12/15	C20/25	C25/30	C30/37	C25/30	C25/30	C30/37	C30/37
<b>Durability tests</b>								
Carbonation resistance	no	no	yes (LV)	yes (LV)	yes (LV)	yes (LV)	no	no
Chloride resistance	no	no	no	no	no	no	yes (LV)	yes (LV)
Freeze thaw resistance					yes (m)	yes (h)	yes (m)	yes (h)
ASR resistance	no	no	no	(yes (LV))	yes (LV)	yes (LV)	yes (LV)	yes (LV)
Sulfate/chemical resistance	no	no	no	Only , if required				

Water conductivity / penetration is required in some cases (underpass or parts of buildings in ground water).

# Introduction of Swiss durability tests

FPC: factory production control

Property	Document, Test	Remarks
Water conductivity (WL)	SIA 262/1, Annex A	(1968: Water penetration) 1989: Introduction 2003: FPC (2013: Water penetration)
Carbonation resistance	SIA 262/1, Annex I	2013: Introduction, incl. FPC
Chloride resistance	SIA 262/1, Annex C	2003: Introduction, incl. FPC
Frost resistance	SIA 162/1 (2003 withdrawn, but still used)	1968: Introduction 1989: (adopted)
Freeze thaw resistance	SIA 262/1, Annex C	1989: Introduction 2003: FPC
ASR resistance	SIA 262/1, Annex G	2012: Introduction (SBB: 2006; ASTRA: 2007) FPC: every 5 years or in case of important changes of the composition
Sulfate resistance	SIA 262/1, Annex D	2003: Introduction

1968, 1989, 2003: New versions of the Swiss standard for concrete structures (SIA 262)

# Durability of concrete and main influences

RH: Relative humidity, PSD: Pore size distribution

Property	Most important parameters (very simplified)				
	w/c or w/B	w/CaO <sub>reactive</sub>	w/SiO <sub>2, reactive</sub>	Entrained air	Na <sub>2</sub> O <sub>eq</sub>
Carbonation resistance	+	+++ RH, PSD			
Chloride resistance	+		+++ and PSD, exposure cond., incl. carbonation		
Freeze thaw resistance	+			+++ minimum temp.	
ASR resistance	+	+	+		++ and reactivity of aggregates and exposure conditions
Leaching resistance	+	++	++		

# Durability of concrete: Main research publications

Property	Publications <sup>1)</sup>	Download <sup>1)</sup>
Carbonation resistance	2019-1, 2018-1, 2012-5	+
Chloride resistance	2002-2, 2017-1, 2016-1a	+
Freeze thaw resistance	2020	+
	2016-1b	-
ASR resistance	2013-1	+
	2006-7	-
Leaching resistance	S. Greve-Dierfeld, fib 2022	Paper available

<sup>1)</sup> Download form website Hunkeler Ingenieurberatung: <https://ingenieurberatung.ch/indexEN.htm#publikationen>

→ Based on the research and experiences a new area with mix designs for concrete without provisions on minimum cement content, w/c ratio, k-values a.s.o. became possible.



**New Annex ND to SN EN 206**

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# Annex ND to SN EN 206

- The purpose of this National Annex ND on performance-based design methods for concrete is to enable more innovation in terms of sustainability and concrete performance.
- Concretes according to national annex ND must fulfil the existing limit values for concrete performance in Switzerland analogous to conventional concretes.
- The composition of the concrete, the type and quantity of cement, type I and II additives and the quantity of water added shall be selected in such a way that all the required fresh and hardened concrete properties are achieved, and the corrosion protection of any reinforcement is ensured.
- No provisions for e.g., minimum cement content, w/c ratio or k-values are given.

sia

prSIA 262.051+A2/ND Bauwesen

 Schweizer Norm  
Norme Suisse  
Norma Svizzera

EN 206:2013+A2:2021/ND

Béton – Spécification, performances, production et conformité – Annexe nationale ND à SN EN 206 :2013+A2 :2021  
Concrete – Specification, performance, production and conformity –National annex ND to SN EN 206:2013+A2:2021

## Beton – Festlegung, Eigenschaften, Herstellung und Konformität – Nationaler Anhang ND zur SN EN 206:2013+A2:2021

### Vernehmlassung Entwurf prSN EN 206:2013+A2:2021/ND:2023-07

Wir bitten Sie, den Entwurf zu prüfen und allfällige Stellungnahmen nach den Ziffern der Norm geordnet einzureichen an: [SN206@sia.ch](mailto:SN206@sia.ch)

Bitte verwenden Sie zu diesem Zweck das elektronische Formular, das Sie unter [www.sia.ch/vernehmlassungen](http://www.sia.ch/vernehmlassungen) finden. Stellungnahmen in anderer Form können wir leider nicht berücksichtigen.

Die Vernehmlassungsfrist läuft bis 27. Oktober 2022

Dieser Entwurf hat keine Gültigkeit und darf nicht angewendet werden.

Referenznummer  
prSN EN 206:2013+A2:2021/ND:2023-07 de

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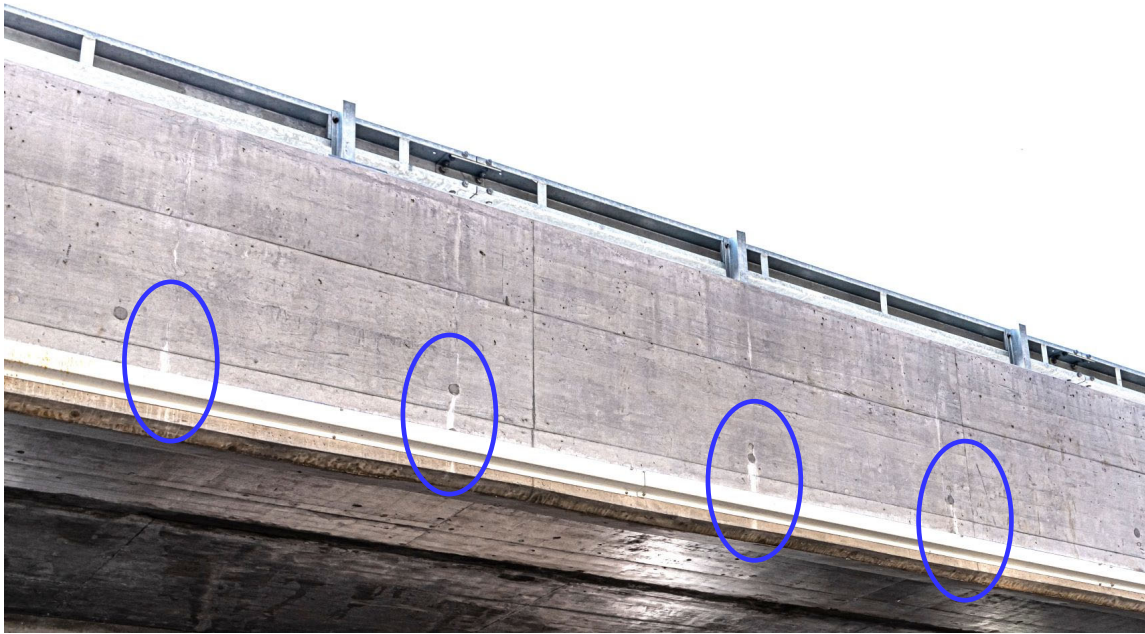
# Annex ND to SN EN 206 (Draft Hunkeler, 2020)

## Concrete requirements to reduce crack risks in lengthy (thick) elements – A pilot application in Tüscherz/Switzerland

FHWA – ASTRA Technology Exchange Concrete Bridges,  
06-13-2023, Ittigen Switzerland

Dr. Fritz Hunkeler  
Hunkeler Ingenieurberatung, Möriken, Switzerland  
<https://ingenieurberatung.ch/>

# Appearance of parapet with standard concrete type G (1½ years after concreting)

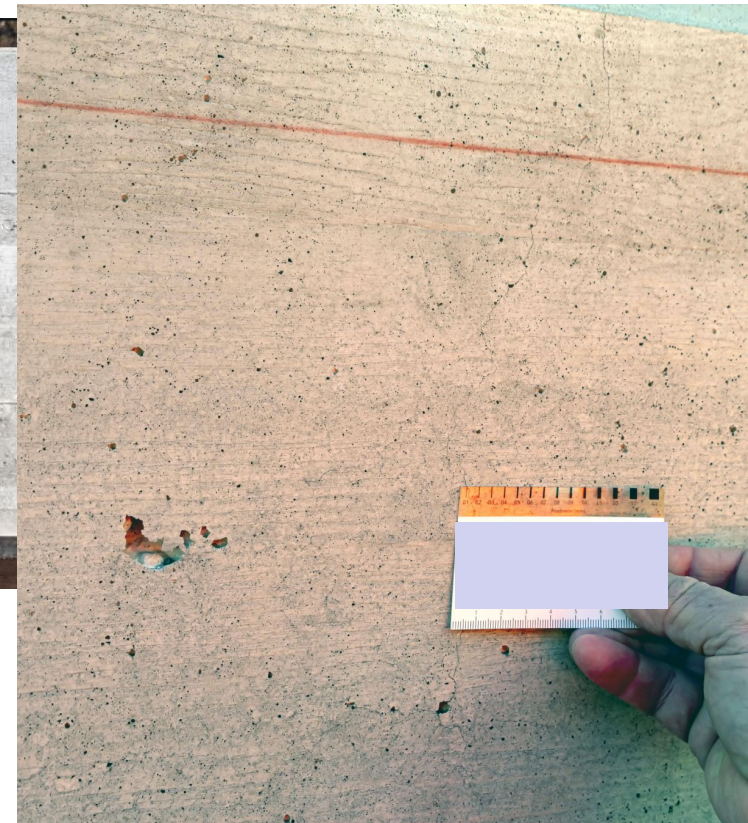


Cracks with crack widths up to 0.3 mm and **lime deposits** at the lower end of the outside

# Appearance of parapet with new concrete type G-ND (1½ years after concreting)



Fine crack pattern with crack widths of 0.1 up to max. 0.2 mm and **no lime deposits** on the outside,



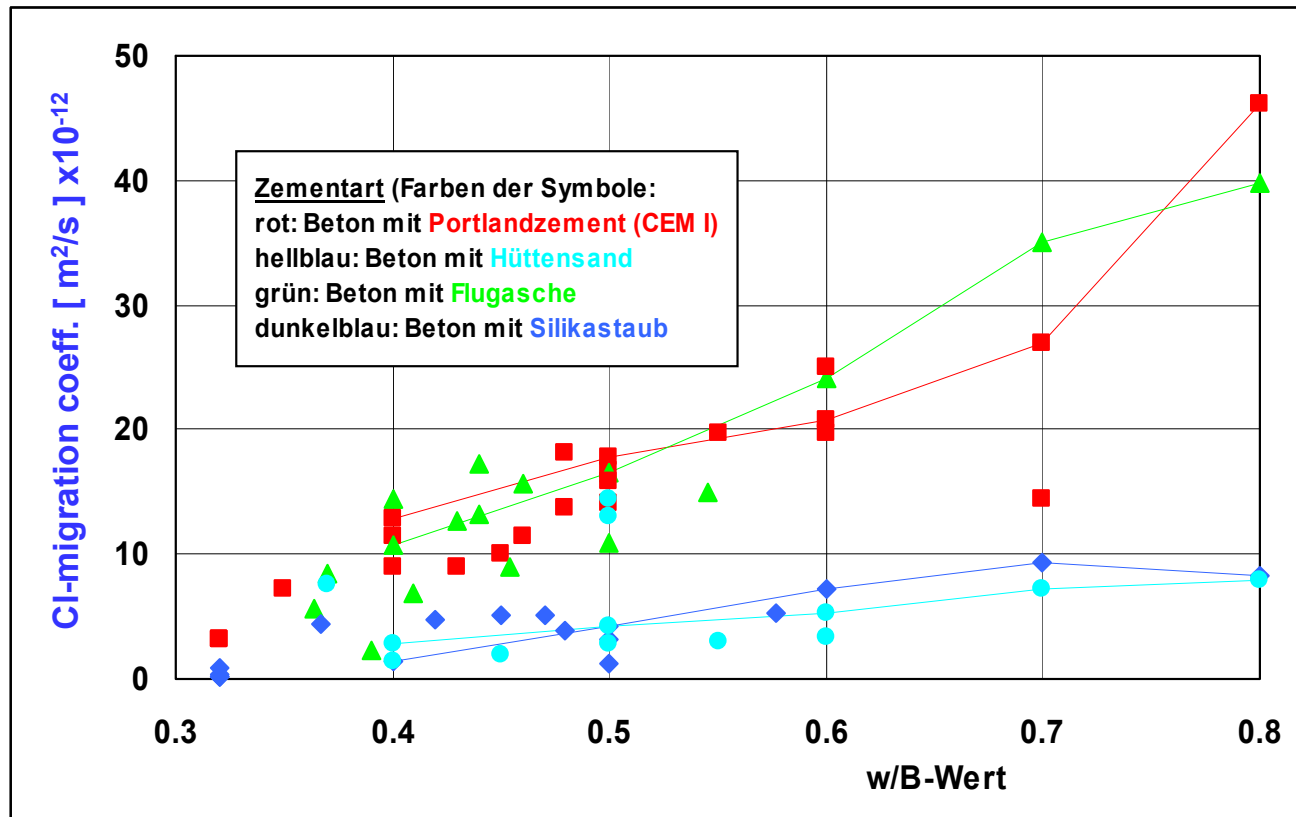
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# Durability tests: Major problems to be solved

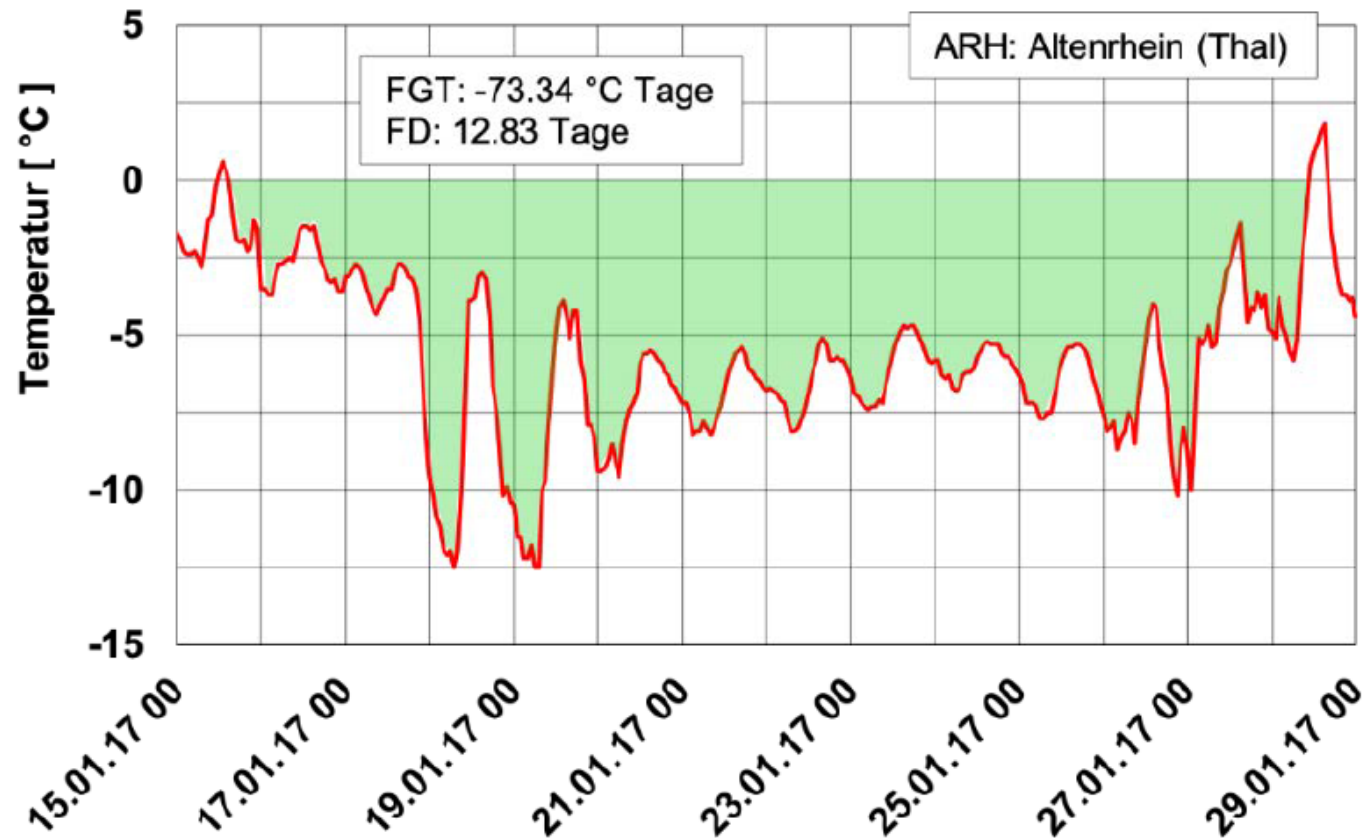
- Carbonation resistance (lab test and modelling): Influence of RH and its dependency of concrete composition, i.e. cement type, w/c ratio. New cement types might cause premature reinforcement corrosion.
- Chloride resistance (lab test and modelling): aging factor, combination with carbonation.
- Freeze Thaw resistance: details of test procedure: minimum temperature, number and duration of cycles, missing link between lab results and field experiences. Large scatter. No models available.
- ASR resistance: long term efficiency of current measures, missing field measurements of younger structures. No models available (no “safe” correlation to  $\text{Na}_2\text{O}_{\text{eq}}$ ).

# Durability test: Chloride resistance



It was a long way for engineers (clients, designers) to accept that CEM I is by far not the best solution for XD3 exposure conditions!

# Durability test: Freeze-thaw resistance



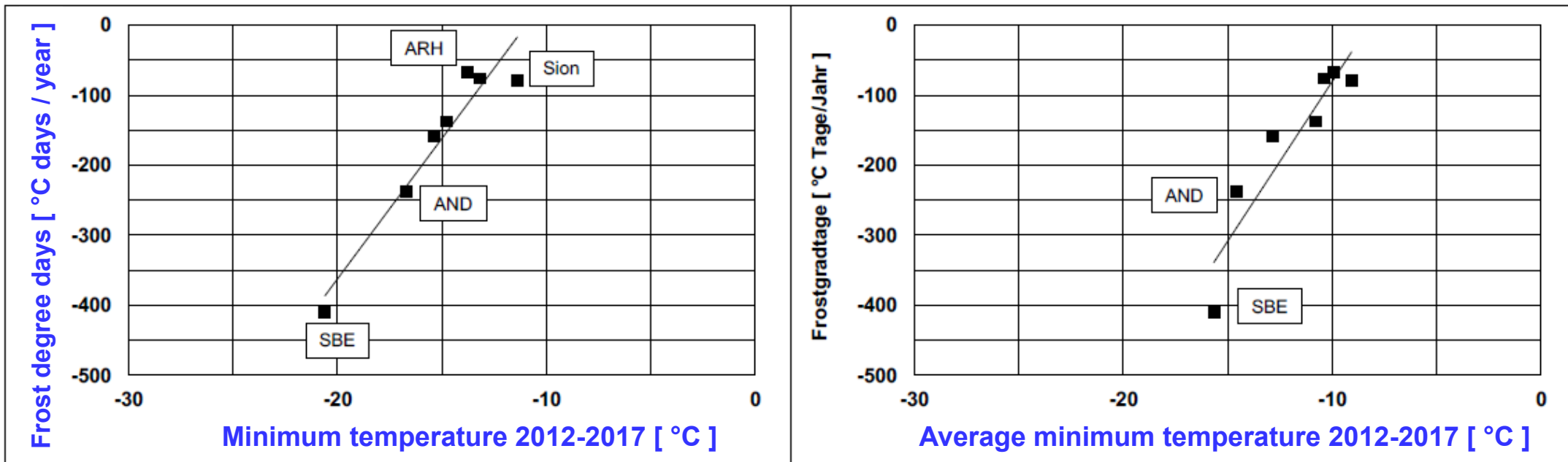
For a very long time the number of cycles with crossing 0 °C was considered as a main factor. But this number is not a critical parameter.

It is better to use:

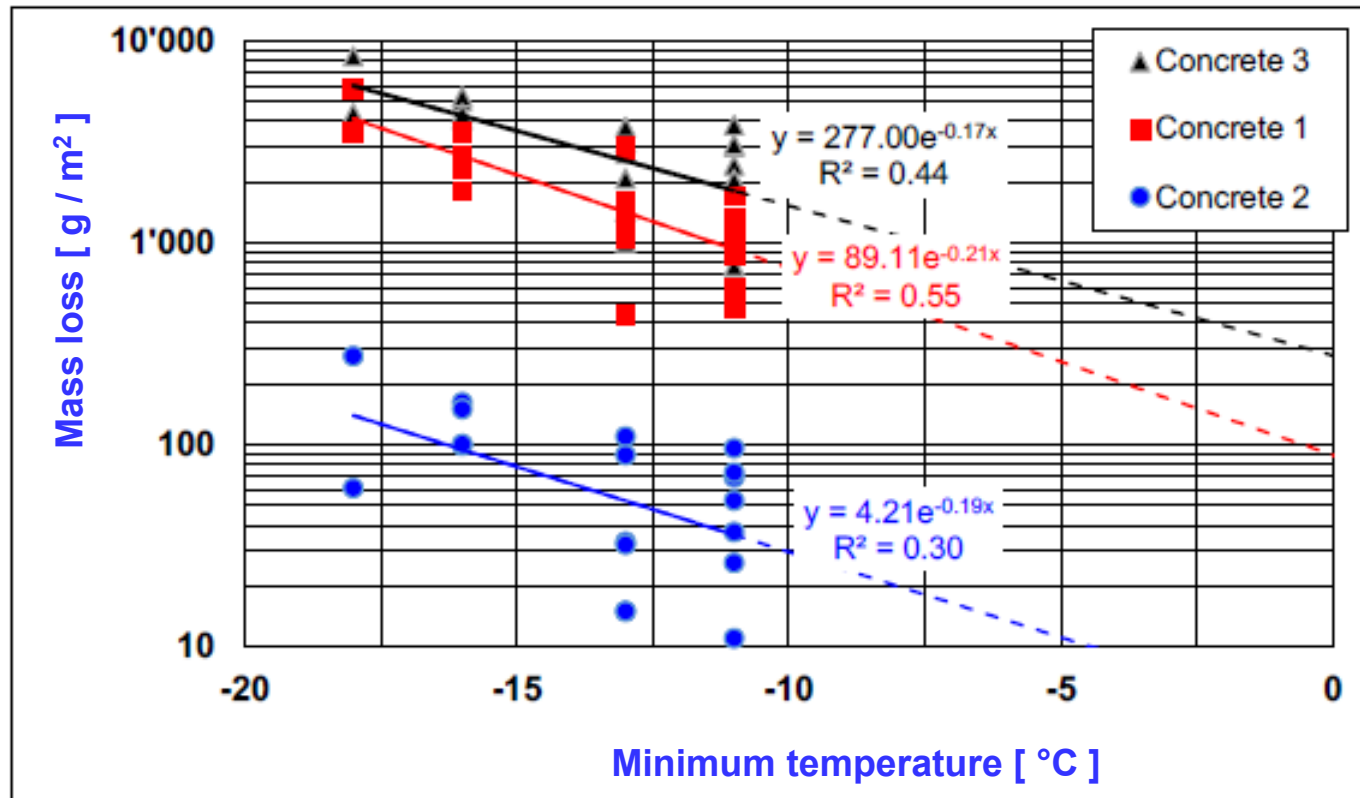
- Frost degree days (FGT): green area under curve.
- Minimum temperature
- Frost duration (FD)



# Durability tests: Freeze-thaw resistance

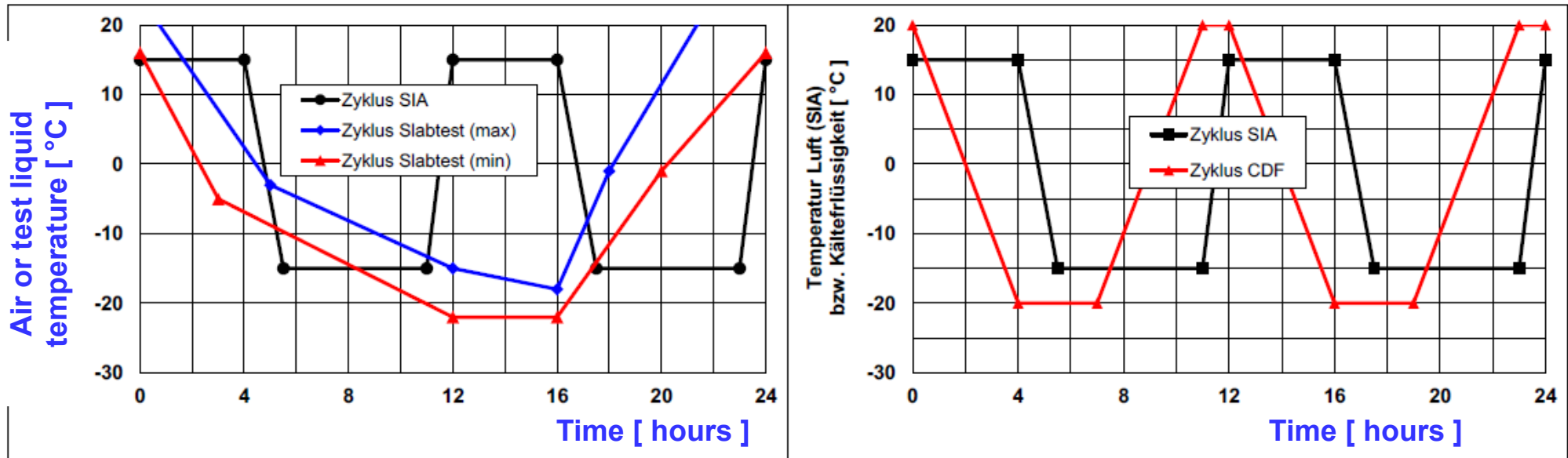


# Durability tests: Freeze-thaw resistance

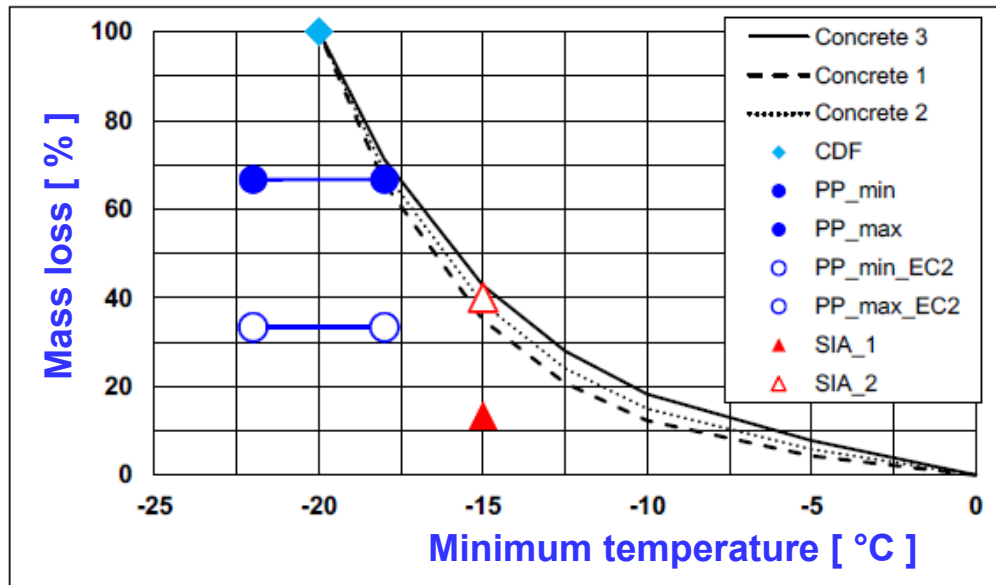


**Correlation:**  
The lower the minimum temperature during testing is, the higher is the mass loss.

# Durability tests: Test cycles SIA, CDF and slab test



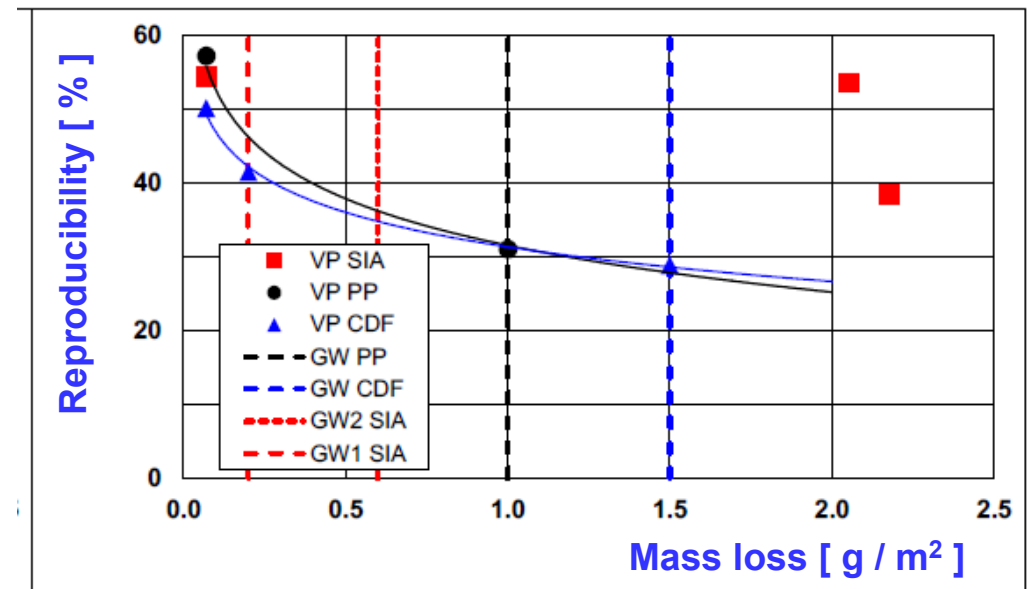
# Durability tests: Test cycles SIA, CDF and slab test



The lower the minimum temperature during testing, the higher the mass loss.

The limiting values of the three tests (CDF, Slab test, SIA) fit quite well, except the new Swedish proposals (PP\_...\_EC2).

The reproducibility depends on the rigour of the test: (SIA < slab test < CDF)



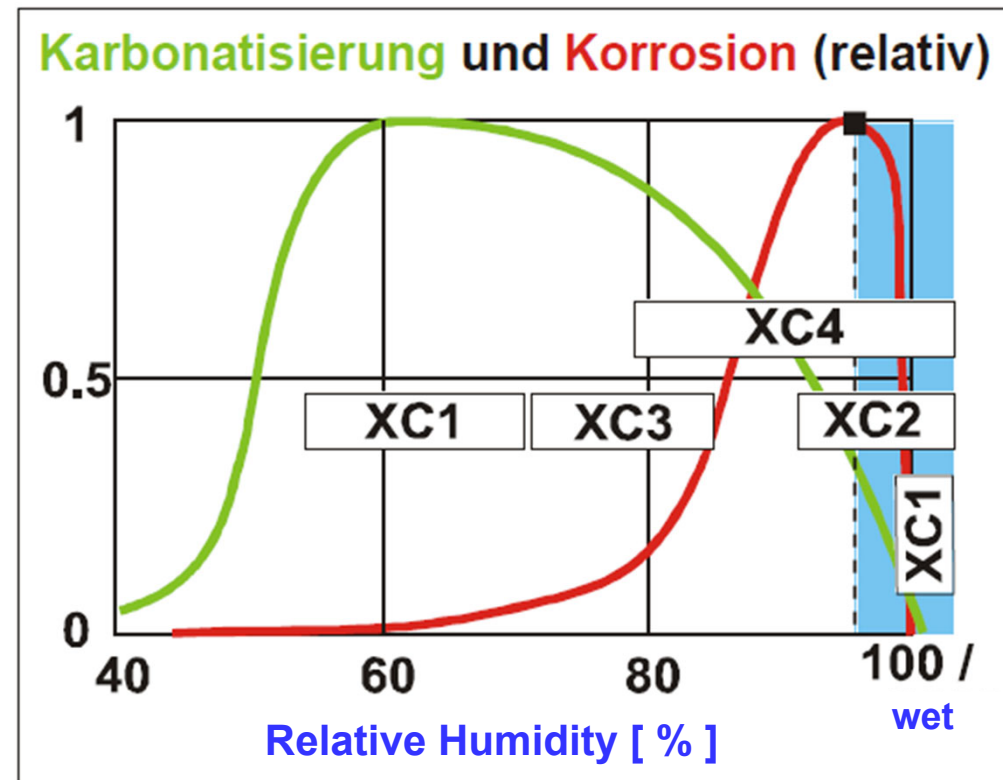
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# Carbonation and corrosion

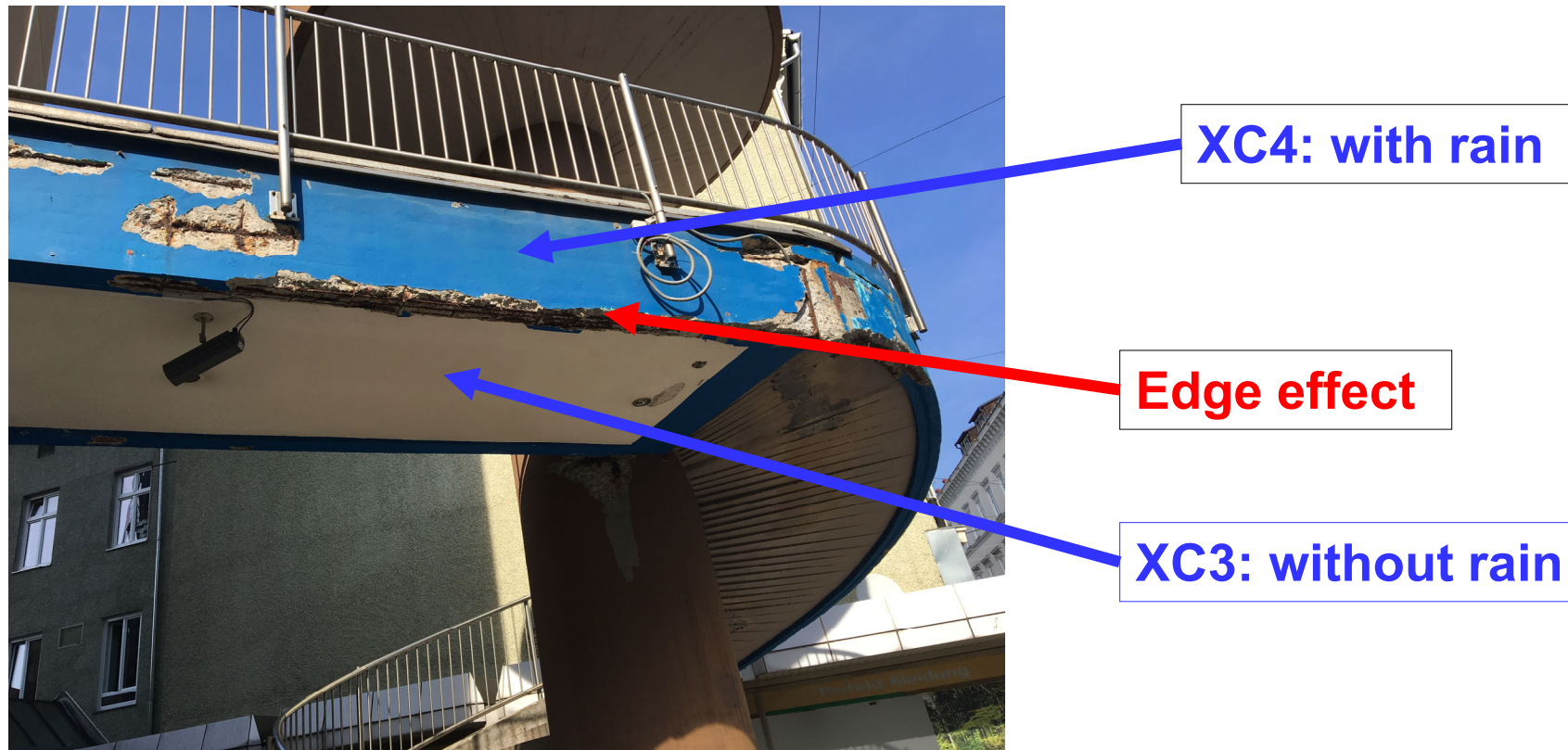
## Difference between XC3 and XC4

- XC3: Carbonation is rather fast, corrosion rate is rather low
- XC4: Carbonation is rather slow, corrosion rate is rather high



# Carbonation and corrosion

## Difference between XC3 and XC4



# Carbonation – Laboratory and field studies

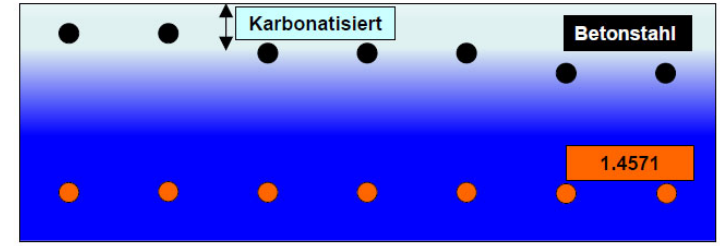
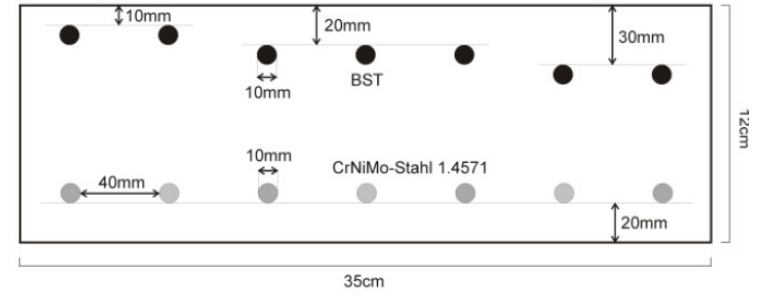
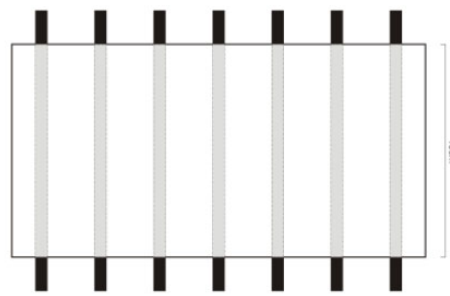
Long term carbonation measurements at three sites in “Stevenson Screens”  
Simulation of XC3, i.e., sheltered exposure conditions





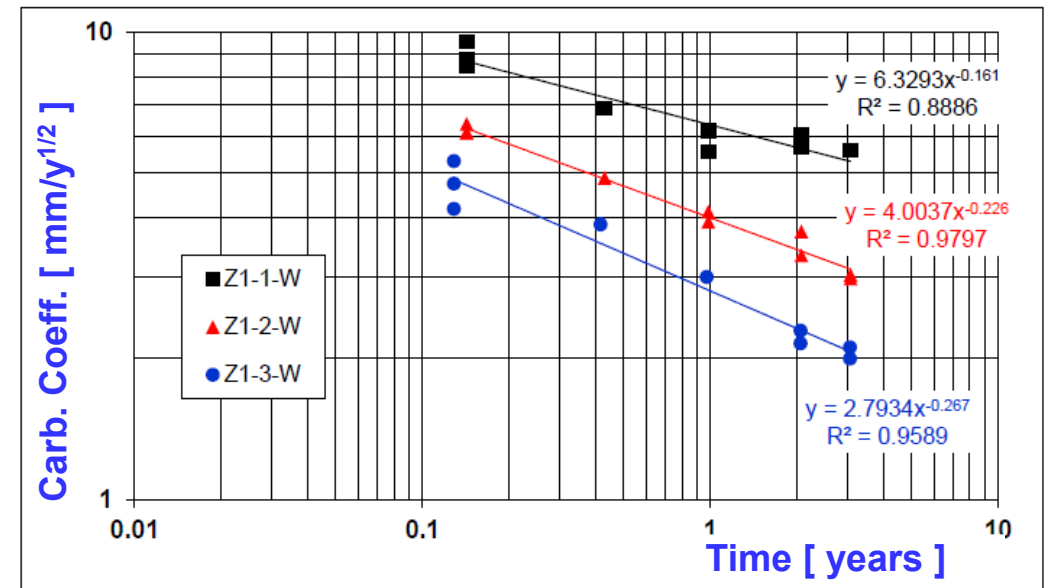
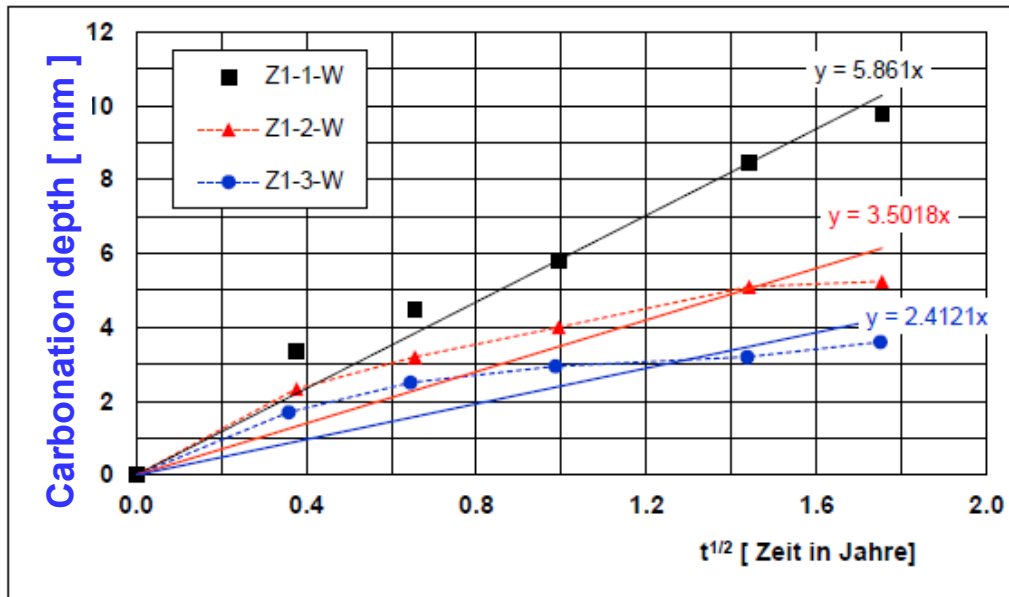
# Carbonation – Laboratory and field studies

- Long term corrosion measurements



Water was periodically poured on the surface to simulate XC4

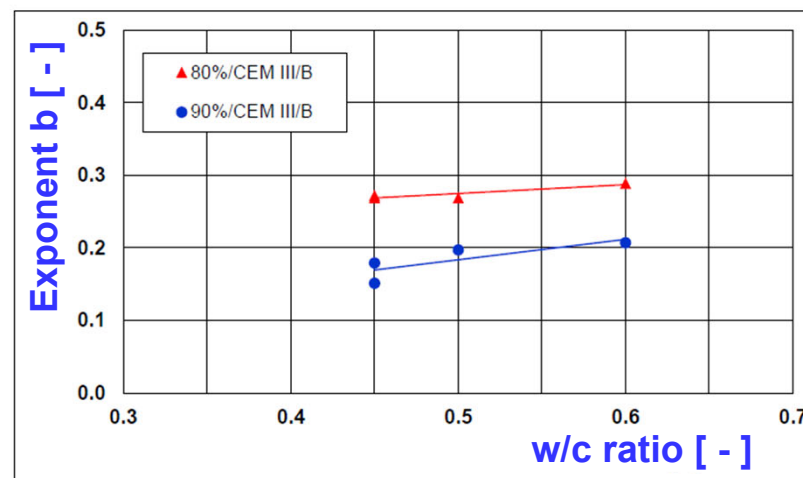
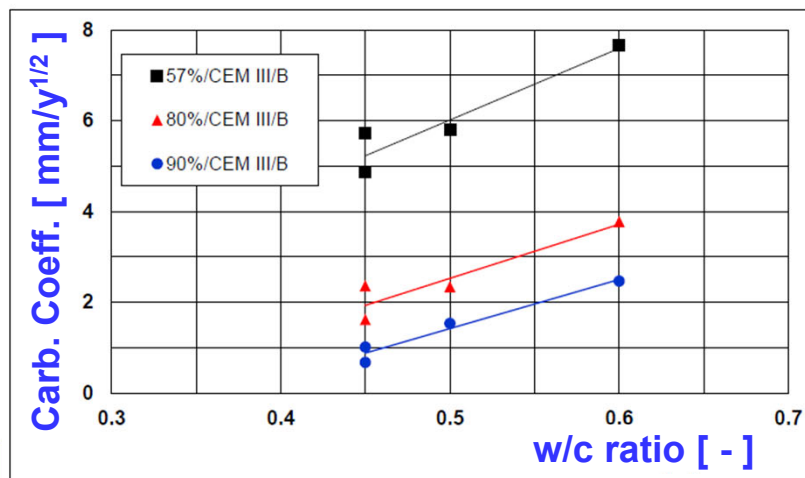
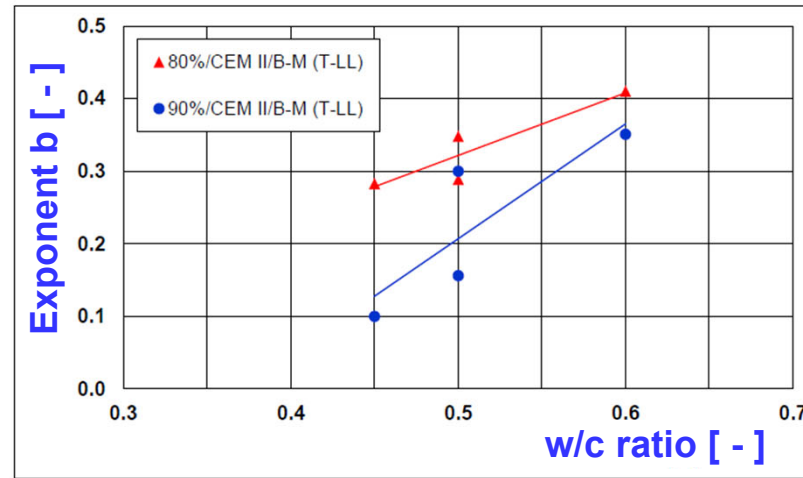
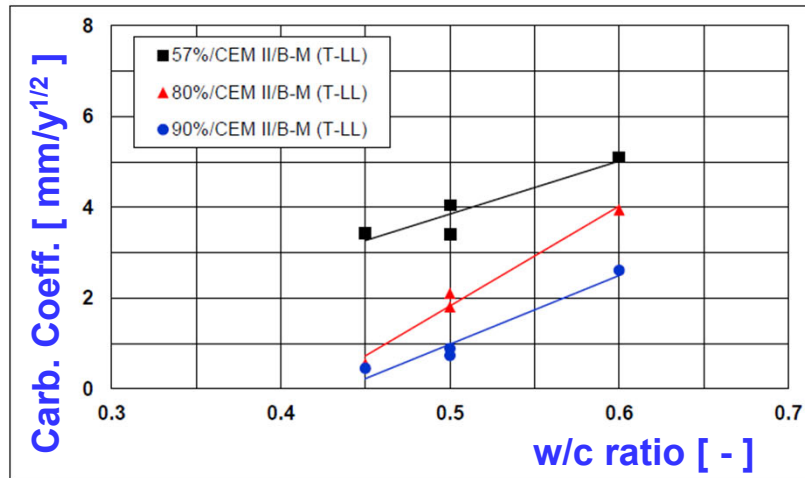
# Carbonation rate under sheltered conditions (XC3)



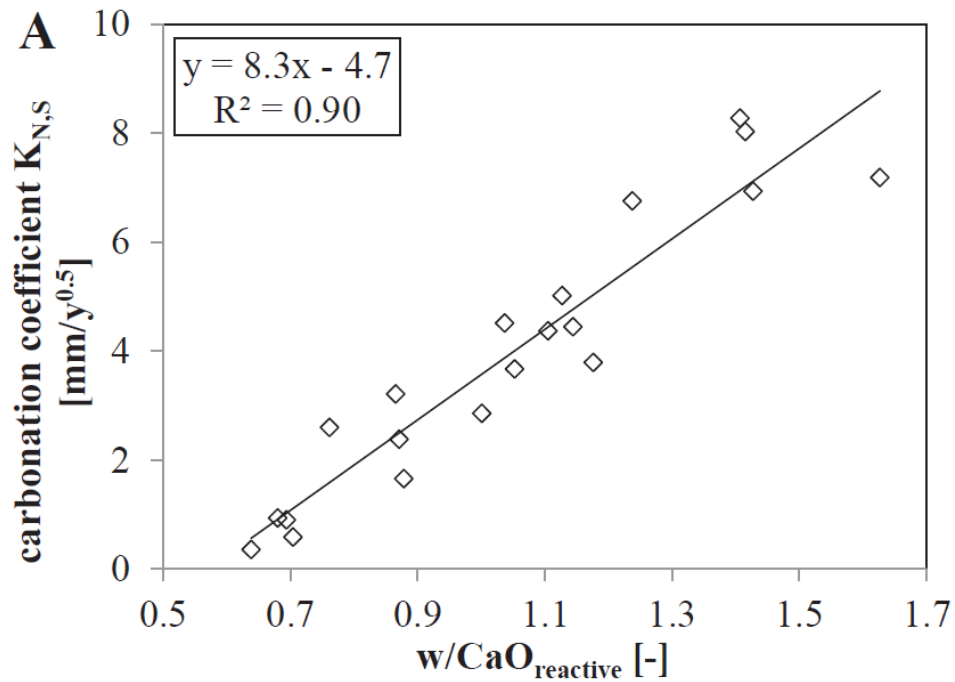
→  $\sqrt{t}$ -law is not valid under humid conditions (RH > 60/65%)

→ Carbonation coefficient decreases with time (aging)

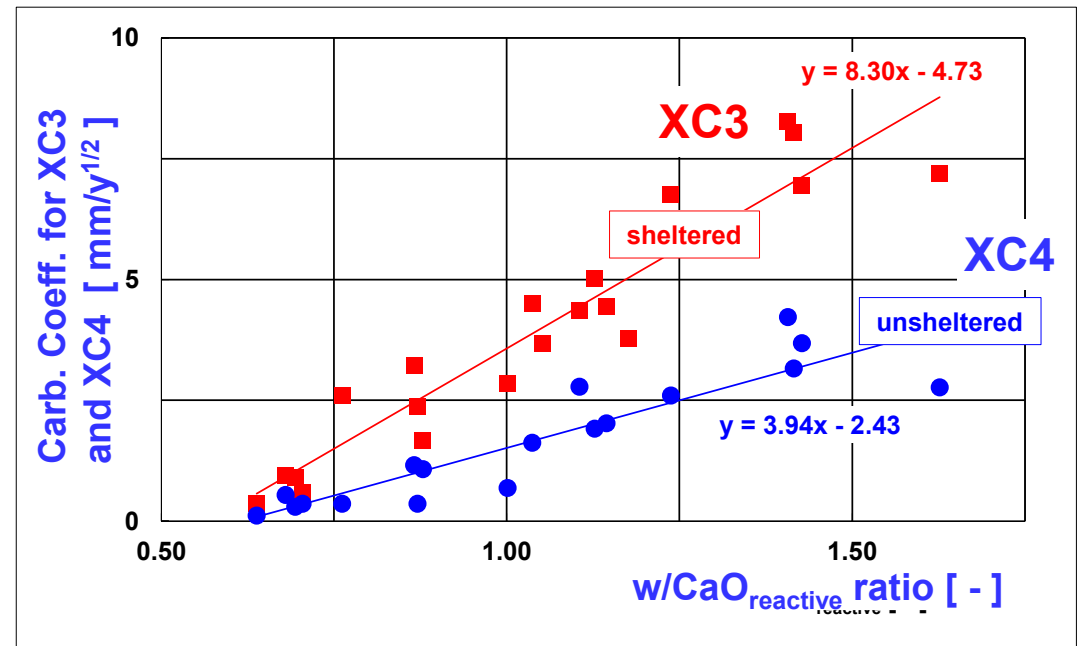
# Carbonation rate under sheltered conditions (XC3)



# Carbonation rate: w/CaO as an auxiliary parameter

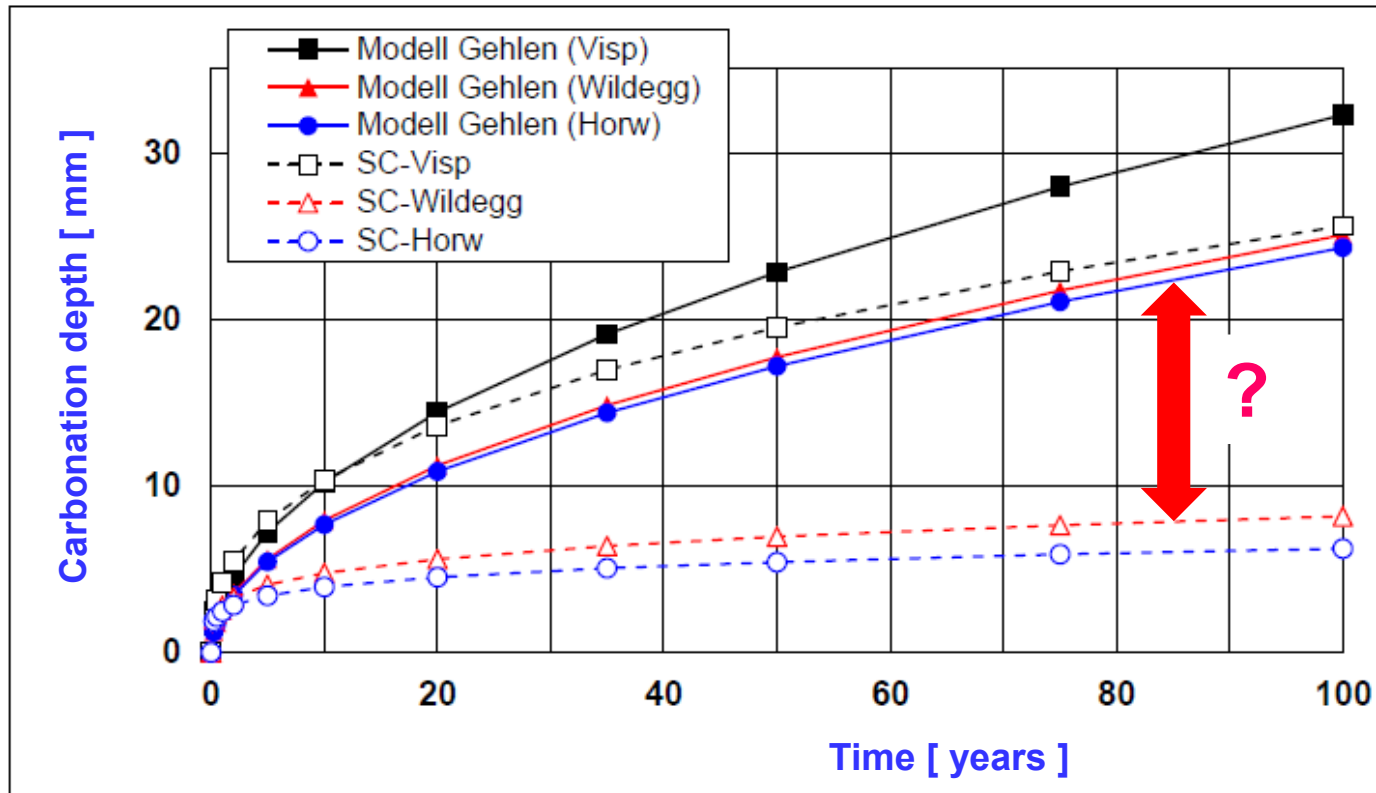


Leemann 2018



Data from Leemann et al. 2015, 2017

# Carbonation rate – Modelling

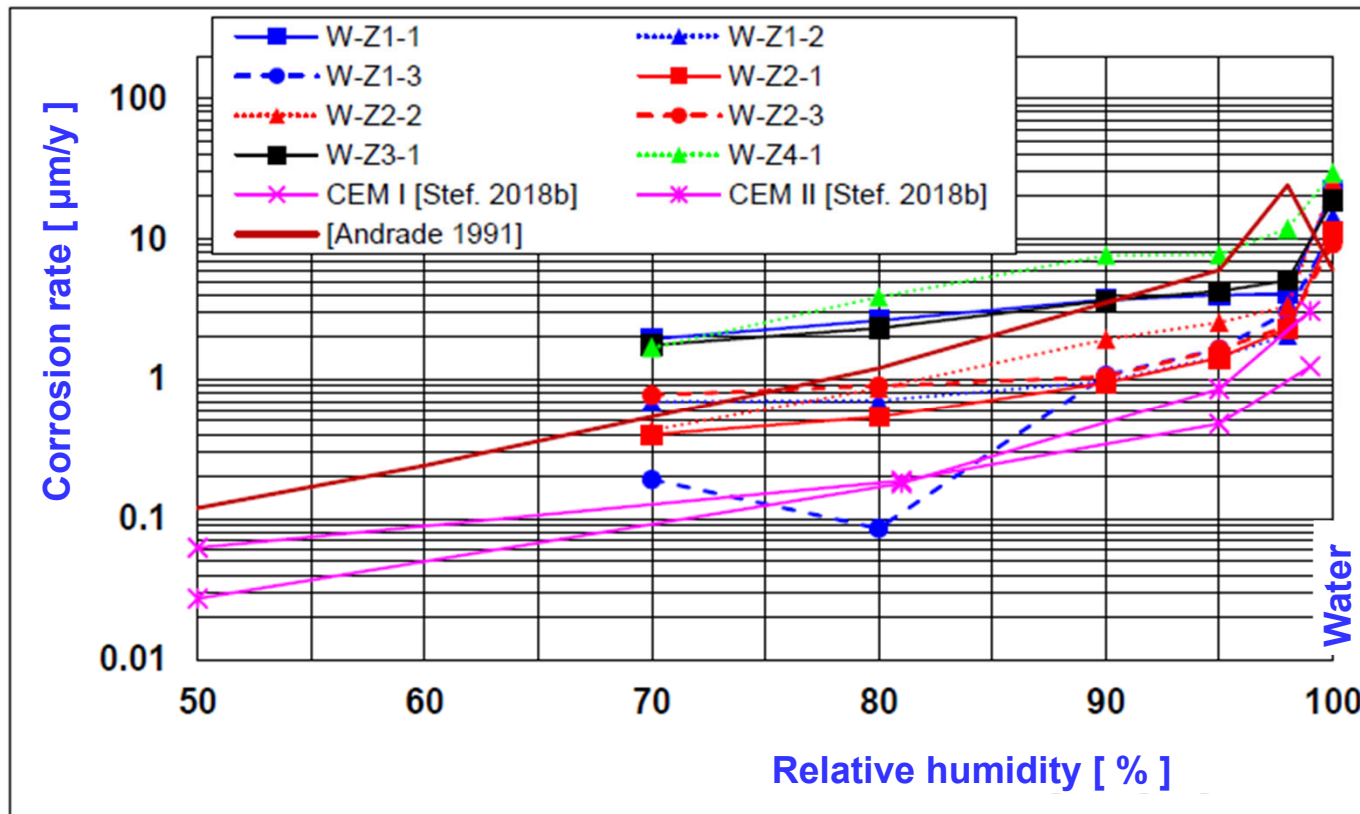


Relative humidity:

- Visp: 65-70%
- Horw: 75-80%
- Wildegg: 75-80%

→ Model Gehlen is much too conservative in some cases!

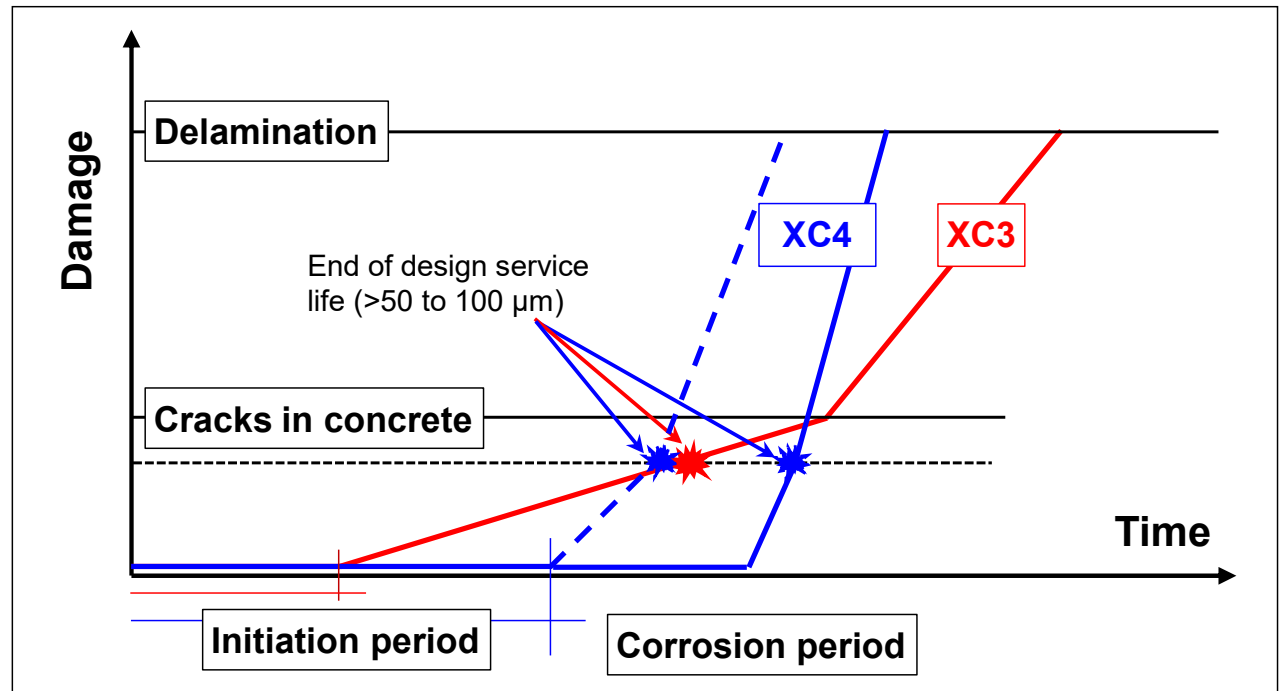
# Corrosion rate under sheltered conditions



RH, %	Average corr. rate, $\mu\text{m/y}$	COV, %
70%	1.0	66
80%	1.5	82
90%	2.6	84
95%	3.1	67
98%	4.3	71
100% (water)	17.8	39

# Difference between XC3 and XC4

- The decisive parameter is the relative humidity!
- ➔ Under humid-wet condition ( $R > 80\%$  and exposed to rain) the carbonation rate dominates the service life since under such conditions the corrosion rate is fast and the corrosion period leading to cracks is short and might not be considered.



# Content

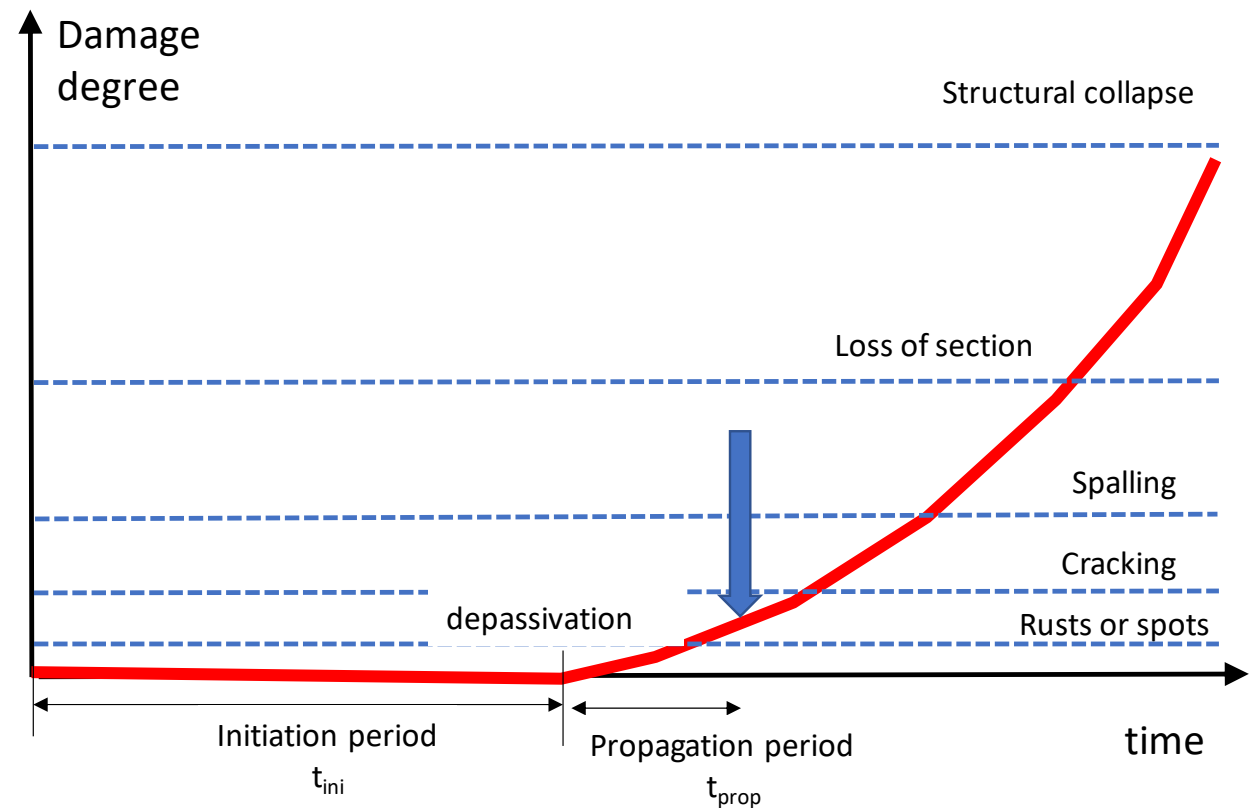
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# Eurocode 2 - Design of concrete structures

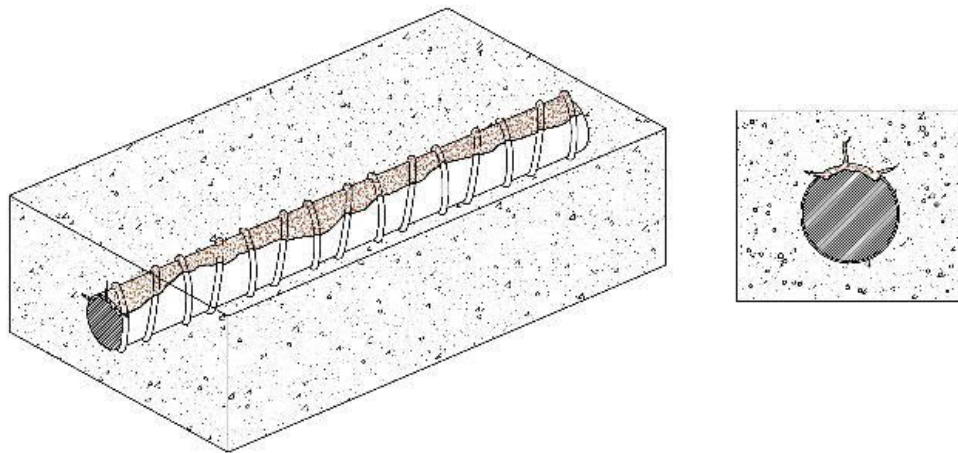
## Modelling including the corrosion process

- Corrosion period is taken into account for evaluation the concrete covers
- Important for XC3



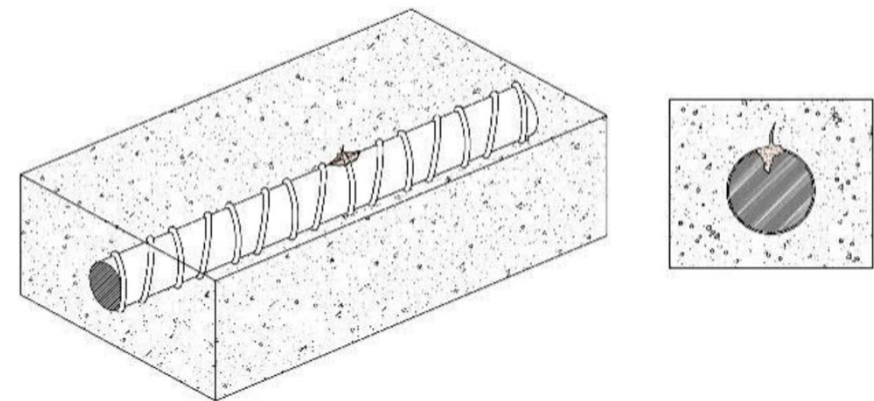
# Schematic illustration of the limit state regarding general corrosion and localized corrosion

A) General surface corrosion (carbonation)



End of service life: crack width of 50  $\mu\text{m}$ .

B) Localised pitting corrosion (chloride attack)



End of service life: pit depth of 500  $\mu\text{m}$ .

# Eurocode 2 - Design of concrete structures

Part 1-1: General rules and rules for buildings, bridges and civil engineering structures (April 2023)

<b>6</b>	<b>Durability and concrete cover.....</b>	<b>79</b>
<b>6.1</b>	<b>General.....</b>	<b>79</b>
<b>6.2</b>	<b>Requirements for durability .....</b>	<b>79</b>
<b>6.3</b>	<b>Environmental exposure conditions.....</b>	<b>80</b>
<b>6.4</b>	<b>Exposure resistance classes .....</b>	<b>84</b>
<b>6.5</b>	<b>Concrete cover.....</b>	<b>85</b>
<b>6.5.1</b>	<b>Nominal cover .....</b>	<b>85</b>
<b>6.5.2</b>	<b>Minimum cover .....</b>	<b>85</b>
<b>6.5.3</b>	<b>Allowance in design for deviation in cover.....</b>	<b>89</b>

- Background Document for prEN1992-1-1:2020, section 6 Durability,
  - CEN/TC 250/SC 2/WG 110 N 350, Draft 2021-06-09 /MH
  - CEN/TC 250/SC 2 N2087, 31. March 2023

# Eurocode 2 - Design of concrete structures

## Concrete cover

$$c_{\text{nom}} = c_{\text{min}} + \Delta c_{\text{dev}} \quad \Delta c_{\text{dev}} \text{ allowance for deviation; see Table 6.7 (NDP)}$$

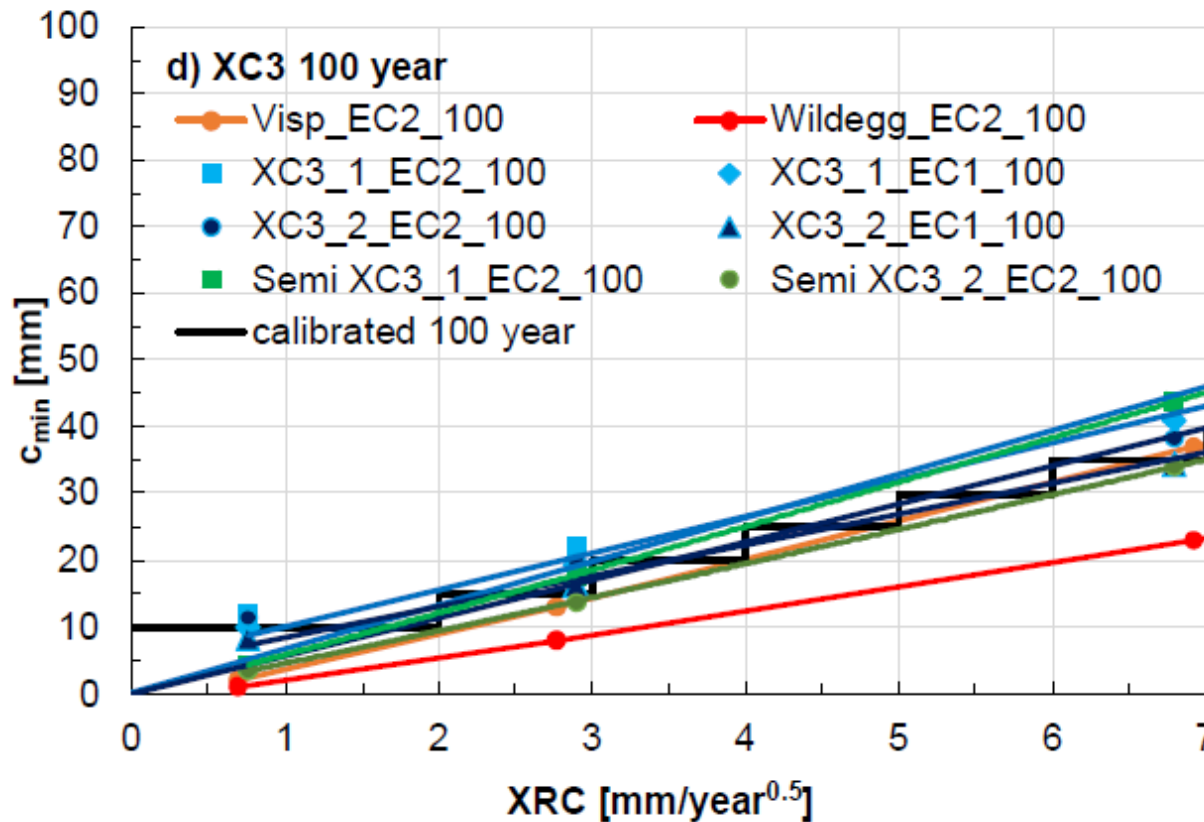
$$c_{\text{min}} = \max \{ c_{\text{min,dur}} + \Sigma \Delta c; c_{\text{min,b}}; 10 \text{ mm} \}$$

**Table 6.5 — Minimum cover  $c_{\text{min,b}}$  for reinforcing steel**

Steel type	$c_{\text{min,b}}^{\text{a}}$
Separated bars	Diameter of bar
Bundled bars	Equivalent diameter $\phi_b$ (see 11.4.3)
<sup>a</sup> Where the specified maximum aggregate size $D_{\text{upper}}$ is $> 32$ mm, the minimum cover $c_{\text{min,b}}$ should be increased by 5 mm.	

# Eurocode 2 – Concrete cover $c_{min,dur}$

Modelling in TG 10 of CEN/TC 250/SC 2/WG 1 - Discussions



- Modelling by a French, Danish, Spanish und Swiss group
- ➔ To reach a consensus was very difficult.

# Concrete cover $c_{\min,dur}$ in dependency of ERC and design service life with reliability index $\beta = 1.5$

Table 6.3 (NDP) — Minimum concrete cover  $c_{\min,dur}$  for carbon reinforcing steel — Carbonation

ERC	Exposure class (carbonation)							
	XC1		XC2		XC3		XC4	
	Design service life (years)							
	50	100	50	100	50	100	50	100
XRC 0,5	10	10	10	10	10	10	10	10
XRC 1	10	10	10	10	10	15	10	15
XRC 2	10	15	10	15	15	25	15	25
XRC 3	10	15	15	20	20	30	20	30
XRC 4	10	20	15	25	25	35	25	40
XRC 5	15	25	20	30	25	45	30	45
XRC 6	15	25	25	35	35	55	40	55
XRC 7	15	30	25	40	40	60	45	60

NOTE 1 XRC classes for resistance against corrosion induced by carbonation are derived from the carbonation depth [mm] (characteristic value 90 % fractile) assumed to be obtained after 50 years under reference conditions (400 ppm CO<sub>2</sub> in a constant 65 %-RH environment and at 20 °C). The designation value of XRC has the dimension of a carbonation rate [mm/√(years)].

NOTE 2 The recommended minimum concrete cover values  $c_{\min,dur}$  assume execution and curing according to EN 13670 with at least execution class 2 and curing class 2.

NOTE 3 The minimum covers can be increased by an additional safety element  $\Delta c_{dur,y}$  considering special requirements (e.g. more extreme environmental conditions).

- This is the result of a controversial and lengthy discussion in TG 10.
- CEN members can choose their own way to handle this provisions.

→ National Application Document or National Annex

# Concrete cover $c_{\min, \text{dur}}$ acc. to EC2, Table 6.3: Difference to Swiss calculations

Exposure class	XC1		XC2		XC3		XC4	
	50	100	50	100	50	100	50	100
Design service life (years)								
XRC Class	Difference Swiss calculations / EC2							
XRC 0,5	0	0	0	0	0	0	0	0
XRC 1	0	0	0	0	0	5	0	5
XRC 2	0	5	0	5	5	15	5	10
XRC 3	0	5	5	10	10	15	5	10
XRC 4	0	10	5	10	10	10	5	15
XRC 5	5	15	10	15	5	15	5	15
XRC 6	5	15	10	15	10	20	10	20
XRC 7	5	20	10	20	10	20	10	20

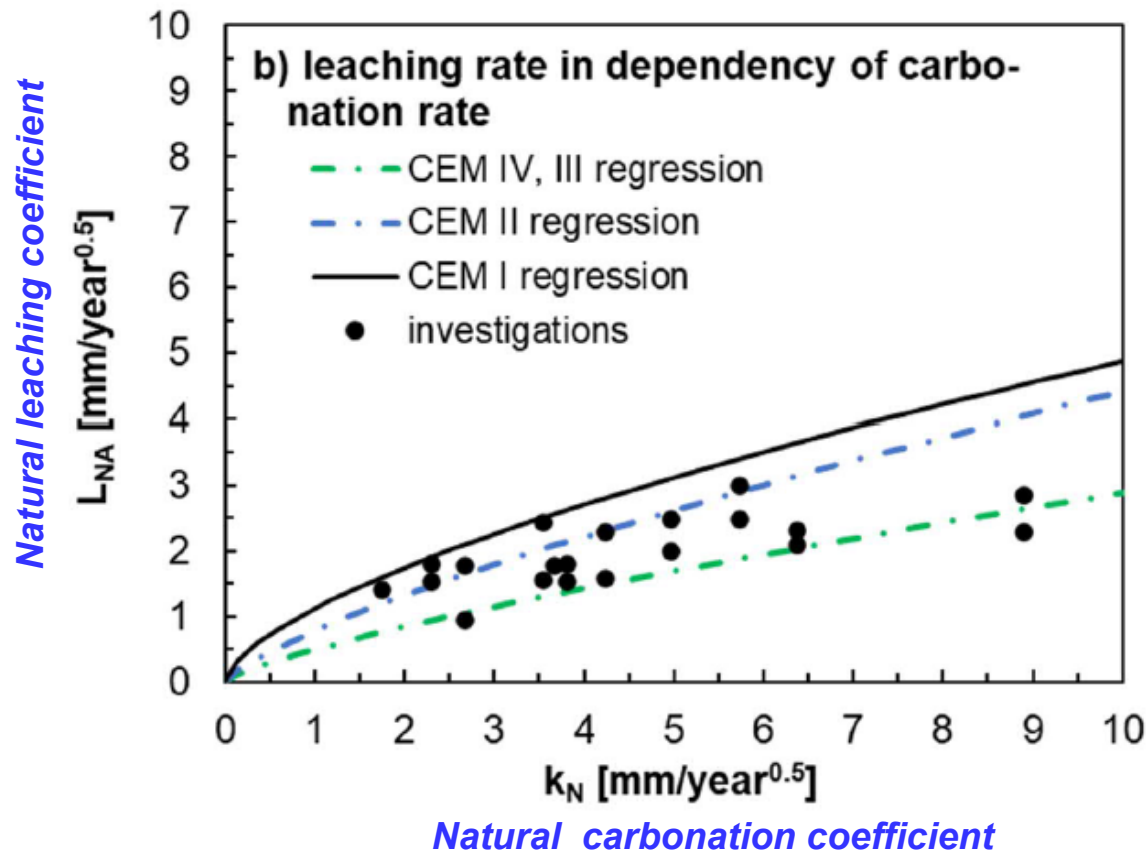
- The values in Table 6.3. are up to 20 mm higher than the Swiss values.
  - Higher covers causes additional costs and environmental load (e.g., CO<sub>2</sub> emissions).
- We need better models!
- Models should be improved based on further experimental evidence, e.g., field studies (lab-field: transfer coefficient).

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# Leaching as a potential risk in the future (XC2)



- Leaching is not taken into account in Table 6.3(NDP).
  - In the future, leaching is going to be more important due to the use low clinker containing concrete.
  - There is no accepted European test procedure.
  - In the meantime, one can use the carbonation resistance test. See diagram on left hand side.
- Increase concrete cover for XC2 by 5-10 mm.

Ref.: S. Greve-Dierfeld, fib 2022

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- Durability tests and limiting values are available (with some uncertainties) for all relevant hardened concrete properties with experiences of more than 10 years.
- This allows new concrete design procedures without requirements on minimum cement content, w/c ratio, k-values etc.
- Currently a new annex to the Swiss concrete standard SN EN 206 is in the enquiry, which should lead to more sustainable concrete recipes.
- There is still a lot to do ... 😊 or ☹️

**Thank you for your attention**

# Fritz Hunkeler – Curriculum vitae

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- 2021 Hunkeler Ingenieurberatung, Möriken, Senior Consultant (concrete, corrosion, durability)
- 2019-2020 Merz Ingenieurberatung GmbH, Möriken, Senior Consultant (concrete, corrosion, durability)
- 2011-2019 TFB AG, Wildegg, Senior Consultant (concrete, corrosion, durability)
- 1994-2011 TFB AG, Wildegg, CEO and Co-owner, Senior Consultant (concrete, corrosion, durability)
- 1984-1994 Swiss Society for Corrosion Protection (SGK), Member of the Executive Board, Consultant
- 1988-2002 Lectureship at ETH Zurich for civil engineers: "Corrosion and Corrosion Protection in Civil Engineering" and "Protection of Reinforced Concrete Structures"
- 1981-1986 ETH Zurich, Institute for Building Materials, Senior Assistant (Prof. Dr. H. Böhni)
- 1981 MIT Cambridge, Boston/USA, postdoctoral study
- 1976-1980 ETH Zurich, IBWK, Assistant and PhD thesis (Prof. Dr. H. Böhni)
- 1975 ETH Zurich, Degree as dipl. Ing. ETH